New Utility Building for







Rockford, Illinois

APPLICABLE CODES:

BUILDING: -2015 ICC

-2015 ICC [IBC] International Building Code (with local amendments)

-2015 ICC [IFC] International Fire Code (with local amendments)

<u>MECHANICAL / PLUMBING:</u>

-2015 ICC [IMC] International Mechanical Code (with local amendments)
-2015 ICC [IFGC] International Fuel Gas Code (with local amendments)
-2014 IDPH [IPC] Illinois Plumbing Code (with local amendments)

ELECTRICAL:
-2014 NFPA 70 [NEC] National Electrical Code (with local amendments)

ENERGY CODE:

-2018 International Energy Conservation Code (with local amendments)

ACCESSIBILTY CODE:

-2018 State of Illinois Accessibility Code-2010 ADA Standards for Accessible Design

LIFE SAFETY CODE:

-2015 NFPA 101 Life Safety Code (with local amendments)

-2010 NFPA 101 Life Safety Code (with local amendments)

OTHERS:

-All those codes & standards [i.e. NFPA 13, NFPA 72, etc.] included by reference by the codes listed above.

 Guide of the American Society of Heating, Refrigerating, and Air Conditioning Engineers (ASHRAE).

2. The standards, regulations, and requirements of OSHA, EPA, National Consumer Protection Agency, and any other public utility servicing the community.

3. Exceptions and amendments to above codes and standards adopted by the

4. Provide fire extinguishers, smoke detectors and all other life safety devices as required by code. All locations to be approved by Blakemore Architects.

5. City of Rockford Zoning Ordinance.

GENERAL NOTES:

- Contractor as agent for the owner, shall apply for and pay for all permits issued in accordance with section 105 of IBC 2015. the application shall be submitted on a written form acceptable to the building official.
- All work shall be conducted, installed and completed in a workmanlike and acceptable manner so as to secure the results intended by the IBC Code (2015).
 This building is equipped with alarm systems for smoke, fire and security.

SHEET INDEX:

ARCHITECTURAL:

A101 Floor Plans 08-03-2020
A102 Elevations 08-03-2020
A103 Wall Sections 08-03-2020

STRUCTURAL:

 S001
 General Notes
 08-03-2020

 S100
 Foundation Plan
 08-03-2020

 S101
 Roof Framing Plan
 08-03-2020

 S300
 Sections & Details
 08-03-2020

CIVIL:

C-001 Legend, General Notes & Location Map-For Reference Use Only
C-002 Site Plan-For Reference Use Only
08-07-2020
08-07-2020

CODE SYNOPSIS:

Construction Type:
Use Group

S-1 (Unconditioned Storage)

Fully Sprinkled None (less than 12,000 s.f.)

Allowable Size 15,750 s.f. (9,000 s.f. plus 6,750 s.f. perimeter increase)

Building Size 10,404



PROFESSIONAL DESIGN FIRM REGISTRATION # 184-003342



7-16-2020 date

license expires 11-30-20



ARCHITECTS
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Email: Brian@blakemore-architects.com
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New Utility Building

for



3501 Kishwaukee Street Rockford, Illinois

THE CONTRACTOR SHALL DETERMINE EXACT DIMENSIONS AND CONDITIONS AT THE SITE PRIOR TO SUBMITTING A BID. THE CONTRACTOR SHALL COORDINATE ALL DRAWINGS WITH ACTUAL FIELD CONDITIONS PRIOR TO PROCEEDING WITH THE WORK AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES. THIS DRAWING IS THE PROPERTY OF BLAKEMORE ARCHITECTS AND MAY NOT BE REPRODUCED WITHOUT THE PRIOR WRITTEN PERMISSION OF THE ARCHITECT.

NO	. DATE	DESCRIPTION	
1.	06-26-2020	Initial Layout	
2.	07-7-2020	Review Set	
3.	07-16-2020	Review Set	
4.	08-03-2020	Issued for Construction	

C COPYRIGHT Blakemore Architects 20

BA Project No. 20-18

Scale

3/32" = 1'-0"

Sheet Title

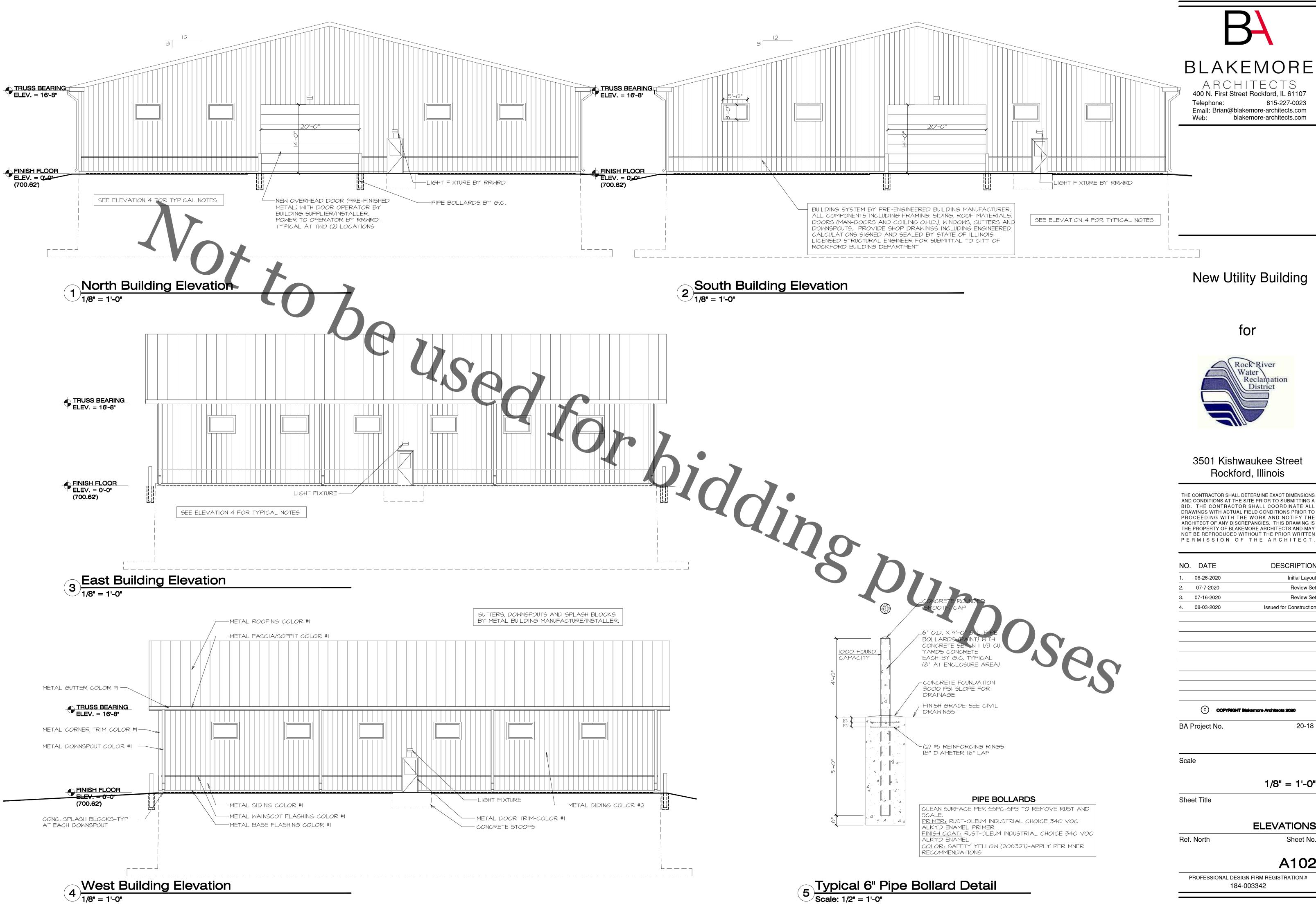
FLOOR PLANS



A101

Sheet No.

PROFESSIONAL DESIGN FIRM REGISTRATION # 184-003342



ARCHITECTS 400 N. First Street Rockford, IL 61107 815-227-0023 Email: Brian@blakemore-architects.com blakemore-architects.com

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20-18 BA Project No.

1/8" = 1'-0"

ELEVATIONS

Sheet No.

PROFESSIONAL DESIGN FIRM REGISTRATION # 184-003342



BLAKEMORE

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New Utility Building

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Initial Layout
Review Set
Review Set
ssued for Construction



BA Project No.

Scale

3/4" = 1'-0"

Sheet Title

WALL SECTIONS

Ref. North

A103

Sheet No.

PROFESSIONAL DESIGN FIRM REGISTRATION # 184-003342

GENE	RAL NOTES			
1.1	GENERAL:			
1.1.1	SCOPE: THE FOLLOWING GENERAL AND S CONTRACTORS AND SUPPLIERS ENGAGED DOCUMENTS. THESE NOTES SUPPLEMENT SPECIFICATIONS.	IN EXECUTION OF	F THE WORK SHOWN	ON THE CONTRACT
1.1.2	REFERENCES: ALL CONSTRUCTION SHALL PLANS AND SPECIFICATIONS GOVERNING LOCAL AND MUNICIPA 2015 INTERNATIONAL BUILDING ASCE 7-10 ASTM INTERNATIONAL (ASTM) OCCUPATIONAL SAFETY AND HEAL CONCRETE CONSTRUCTION: AMERICAN CONCRETE INSTITUTE CONCRETE REINFORCING STEEL PRECAST/PRESTRESSED CONCRETE WOOD CONSTRUCTION & TRUSSES: NATIONAL DESIGN SPECIFICATIO AMERICAN INSTITUTE OF TIMBER APA - THE ENGINEERED WOOD AS TRUSS PLATE INSTITUTE (TPI) CONTRACTOR SHALL ENSURE FAMILIAR OBSERVATIONS WILL BE IN CONFORMA	L CODES CODE (IBC) TH ADMINISTRAT: (ACI) NSTITUTE (CRSI: INSTITUTE (PC: N (NDS) FOR WOO CONSTRUCTION (APA: ITY OF THE ABOV	ION (OSHA)) I) DD CONSTRUCTION (AITC)) VE ITEMS. INSPE	
1.1.3	DESIGN DATA: RISK CATEGORY			CATEGORY I
	ROOF DEAD LOADS (D): WOOD FRAMED ROOF. FLOOR LIVE LOADS (L): STORAGE WAREHOUSE LIGHT	VED ROOF		
	INTERNAL PRESSURE COEFFICIENT, COMPONENTS AND CLADDING DESIGN			
		/1	ECTIVE WIND AREA JLTIMATE LOADS)	
	ROOF ZONES* NEG. ZONE 1 (FIELD) NEG. ZONE 2 (PERIMETER) NEG. ZONE 3 (CORNER) POS. ZONE ALL ZONES OVERHANG ZONE 1 & 2 OVERHANG ZONE 3	10 SF -16.8 PSF -29.3 PSF -43.3 PSF 10.6 PSF -34.3 PSF -57.6 PSF	50 SF -15.7 PSF -23.8 PSF -36.8 PSF 10.0 PSF -34.3 PSF -44.5 PSF	100 SF -15.3 PSF -21.5 PSF -33.9 PSF 10.0 PSF -34.3 PSF -38.9 PSF
	WALL ZONES* NEG. ZONE 4 (INTERIOR ZONE) NEG. ZONE 5 (CORNER ZONE) POS. ZONE 4 & 5	10 SF -19.9 PSF -24.6 PSF	100 SF -17.2 PSF -19.1 PSF	500 SF -15.3 PSF -15.3 PSF 13.7 PSF
	*NOTES: - REFER TO ASCE 7-10, FOR - PLUS AND MINUS SIGNS IND FROM BUILDING SURFACES - PRESSURES MAY BE INTERPO - SEE DRAWINGS FOR NET UPL - "a" = 10% OF LEAST HORIZ BUT NOT LESS THAN	ICATE PRESSURES ESPECTIVELY. LATED BETWEEN TIFT ON JOISTS A ONTAL DIMENSION	THE EFFECTIVE WI AND JOIST GIRDER: N OR 0.4h, WHICH	ND AREAS. S. EVER IS SMALLER
	SEISMIC (E): IMPORTANCE FACTOR, Ie 0.2s MAPPED SPECTRAL RESPONSE A 1.0s MAPPED SPECTRAL RESPONSE A SITE CLASS	ACCELERATION, S ACCELERATION, S IENT, SDS IENT, SD1 FICIENT, R ., OMEGA ACTOR, Cd Cs	Ss	11.2% 6.0% 0.101 0.09 NG FRAME SYSTEM E WOOD WALL WITH OOD SHEAR PANELS 0.0.1 0.0.4.5 0.0.1 0.0.1 0.0.1
1.1.4	DESIGN CRITERIA:		40	00 805
	GEOTECHNICAL REPORT: REFER TO GEOTECHNICAL ENGINEER	ING EXPLORATION	N AND ANALYSIS,	PREPARED BY
	ILLINOIS DRILLING & TESTING, R CONCRETE (NORMAL WEIGHT UNO): FOOTINGS AND SUB-SLABS CIP WALLS AND PIERS INTERIOR SLAB ON GRADE EXTERIOR SLABS TOPPING SLABS GROUT: UNDER BASE PLATES, ASTM C1107 REINFORCEMENT STEEL: #3 BARS & LARGER, ASTM A615 GR #3 BARS & LARGER WELDABLE, AST	GRADE A, B, OR	C	f'c = 3000 PSI f'c = 4000 PSI f'c = 4000 PSI f'c = 3500 PSI f'c = 4000 PSI f'c = 7000 PSI
	WELDED WIRE REINFORCEMENT, AST STRUCTURAL STEEL: ANGLE SHAPES & PLATES, ASTM A3 WIDE FLANGE SHAPES, ASTM A992	M A1064		$F_y = 65000 \text{ PS}$ $F_y = 36000 \text{ PS}$ $F_y = 50000 \text{ PS}$
	HSS TUBE SHAPES, ASTM A500 GRA HSS ROUND SHAPE, ASTM A500 GRA STEEL PIPE, ASTM A53 GRADE B . CHANNEL SHAPES, ASTM A36 M, S AND HP SHAPES, ASTM A572 WOOD CONSTRUCTION: (MINIMUM STRE	DE B		$F_y = 42000 \text{ PSI}$ $F_y = 35000 \text{ PSI}$ $F_y = 36000 \text{ PSI}$ $F_y = 50000 \text{ PSI}$
	RAFTERS - SPRUCE-PINE-FIR		E	$F_V = 135 PSI$ = 1,500,000 PS . $F_b = 1200 PSI$
	STUDS, PLATES -SPRUCE-PINE-FIR			$F_{v} = 180 \text{ PS}$ = 1,800,000 PS . $F_{b} = 675 \text{ PS}$

HEADED SHEAR CONNECTOR STUDS, ASTM A108 EXPANSION ANCHORS

CONCRETE EPOXY ANCHORS MASONRY EPOXY ANCHORS

SCREW ANCHORS (3/16"-1/4") .

SCREW ANCHORS (3/8"-3/4")

DEFLECTION LIMITS

MEMBERS

ROOF MEMBERS

FLOOR MEMBERS

EXTERIOR MEMBERS

LINTELS/HEADER/BEAM MEMBERS

SUPPORTING FLEXIBLE MATERIALS

CURTAIN WALL SPANS 13'-6" OR LESS CURTAIN WALL SPANS GREATER THAN 13'-6"

SUPPORTING RIGID MATERIAL (BRICK, MASONRY, ETC.) NA SUPPORTING METAL WALL PANEL NA

POWDER ACTUATED FASTENERS

STEEL DECKING FASTENERS X-HSN24 BY HILTI, INC. SELF-DRILLING SCREWS TEKS BY ITW BUILDEX

 NUMBERS
 L/360
 L/360
 L/240

 SUPPORTING GYSPUM BOARD CEILINGS
 L/360
 L/360
 L/240

 SUPPORTING FLEXIBLE CEILINGS
 L/360
 L/360
 L/240

 NOT SUPPORTING CEILINGS
 L/240
 L/240
 L/180

 SUPPORTING RIGID MATERIAL (BRICK, MASONRY, ETC.)
 L/600
 L/600
 L/600

 OR MEMBERS
 L/360
 NA
 L/240

SUPPORTING FLEXIBLE MATERIALS
SUPPORTING RIGID MATERIAL (BRICK, MASONRY, ETC.)
SUPPORTING CURTAIN WALLS

L/360 L/360 L/240
L/600 L/600
L/600 L/600
L/175 L/175 L/175 1/8" MAX
OR 3/4"
(CURTAIN WALL

. F_y = 36000 PSI

HIT HY-200 BY HILTI, INC.
HIT HY-70 BY HILTI, INC.

. X-U OR DS BY HILTI, INC.

KWIK-BOLT III BY HILTI, INC.

. KH-EZ BY HILTI, INC.

OR SNOW + LIVE OR SNOW

L/360 NA L/600 NA L/120 NA L/175 NA L/240 NA + 1/4"

WOOD FRAMED ROOF	- ALL SHOP DRAWINGS SHALL CONTAIN THE ISSUE DATE INDICATED ON THE CONSTRUCTION DOCUMENTS, ALONG WITH ANY ADDENDUMS OR REVISION DATES COPIES OF THE STRUCTURAL DRAWINGS SUBMITTED AS SHOP DRAWINGS WILL BE
LIGHT	REJECTED. - ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS SHALL BE NOTED (CLOUD, NOTE, ETC.) ON THE SHOP DRAWINGS SUBMITTED FOR APPROVAL. - ANY CHANGES ON RESUBMITTED SHOP DRAWINGS SHALL BE CLOUDED.
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	1.1.8 DEFERRED COMPONENT SUBMITTALS: SHALL BE SUBMITTED BY THE GENERAL CONTRACTOR TO THE ARCHITECT/ENGINEER PRIOR TO CONSTRUCTION. DEFERRED SUBMITTALS SHALL BE PROVIDED FOR THE FOLLOWING COMPONENTS: PRECAST CONCRETE PLANK, PRECAST CONCRETE WALL PANELS, STRUCTURAL PRECAST CONCRETE, TILT-UP WALL PANELS, METAL BUILDING, STEEL JOISTS AND GIRDERS, COLD-FORMED STEEL, WOOD TRUSSES, AND GLUE LAMINATED LUMBER. NOTES:
EXPOSURE CATEGORY	- GENERAL CONTRACTOR SHALL REVIEW AND STAMP THE DEFERRED SUBMITTALS BEFORE SUBMITTAL TO ARCHITECT/ENGINEER SUBMIT (4) COPIES OF DRAWINGS AND CALCULATIONS TO ENGINEER. (3) COPIES WILL BE RETURNED WITH ENGINEER'S APPROVAL STAMP ALL COMPONENT SUBMITTALS SHALL BEAR AN ORIGINAL SEAL AND SIGNATURE OF THE COMPONENT DESIGNER ALL SUBMITTED COPIES MUST BE THE FINAL "FIELD USE" SETS WHICH INCLUDES ALL CORRECTIONS MADE DUE TO SHOP DRAWING REVIEW COMMENTS.
ROOF ZONES* NEG. ZONE 1 (FIELD) NEG. ZONE 2 (PERIMETER) NEG. ZONE 2 (PERIMETER) NEG. ZONE 3 (CORNER) NEG. ZONE 3 (CORNER) -49.3 PSF -23.8 PSF -21.5 PSF NEG. ZONE 3 (CORNER) -443.3 PSF -36.8 PSF -33.9 PSF POS. ZONE ALL ZONES 10.6 PSF 10.0 PSF 10.0 PSF 10.0 PSF OVERHANG ZONE 1 & 2 -34.3 PSF -34.3 PSF -34.3 PSF OVERHANG ZONE 3 -57.6 PSF -44.5 PSF -38.9 PSF	1.1.9 SPECIAL INSPECTIONS: AN INSPECTION & TESTING COMPANY SHALL BE RETAINED IN ACCORDANCE WITH THE IBC FOR THE FOLLOWING: - SOILS AND EARTHWORK SUPPORTING FOUNDATIONS AND SLABS CONCRETE TEST CYLINDERS AND STRENGTH TESTING CONCRETE REINFORCEMENT.
WALL ZONES* 10 SF 100 SF 500 SF NEG. ZONE 4 (INTERIOR ZONE) -19.9 PSF -17.2 PSF -15.3 PSF NEG. ZONE 5 (CORNER ZONE) -24.6 PSF -19.1 PSF -15.3 PSF POS. ZONE 4 & 5 18.4 PSF 15.6 PSF 13.7 PSF	- MASONRY PER ACI 530.1-05/ASCE 5-05/TMS 402-05/- SPECIFICATION FOR MASONRY STRUCTURES TABLE 4 - LEVEL B QUALITY ASSURANCE STRUCTURAL STEEL WELDED AND BOLTED CONNECTIONS JOIST AND JOIST GIRDER WELDED AND BOLTED CONNECTIONS METAL DECK FASTENING.
*NOTES: - REFER TO ASCE 7-10, FOR ZONE DIAGRAMS PLUS AND MINUS SIGNS INDICATE PRESSURES ACTING TOWARD OR AWAY FROM BUILDING SURFACES RESPECTIVELY PRESSURES MAY BE INTERPOLATED BETWEEN THE EFFECTIVE WIND AREAS SEE DRAWINGS FOR NET UPLIFT ON JOISTS AND JOIST GIRDERS "a" = 10% OF LEAST HORIZONTAL DIMENSION OR 0.4h, WHICHEVER IS SMALLER, BUT NOT LESS THAN EITHER 4% OF LEAST HORIZONTAL DIMENSION OR 3'-0"	- POST INSTALLED EXPANSION AND EPOXY ANCHORS. 1.1.10 CONSTRUCTION LOADS: PLACEMENT OF CONSTRUCTION EQUIPMENT, MATERIALS, AND PERSONNEL SHALL NOT EXCEED THE DESIGN LIVE LOAD OF THE STRUCTURE. CONCRETE SHALL CURE A MINIMUM OF 7 DAYS BEFORE THE APPLICATION OF CONSTRUCTION LOADS. IN ADDITION, EQUIPMENT PLACED ON FLOORS SHALL ALSO COMPLY WITH THE FOLLOWING: FORKLIFT SPACING BETWEEN WHEELS ON AXLE IS NOT LESS THAN 38" C/C OR SCISSOR LIFT SPACING BETWEEN WHEELS ON AXLE IS NOT LESS THAN 25" C/C.
SEISMIC (E): $ \begin{array}{ccccccccccccccccccccccccccccccccccc$	SLAB-ON-GRADE: ALLOWABLE EQUIPMENT AXLE LOADS SLAB DEPTH FORKLIFT SCISSOR/PLATFORM LIFTS 4" NONE 5,400 LBS 5" 8,600 LBS 6,800 LBS >=6" 11,400 LBS 9,200 LBS MODULUS OF SUBGRADE, k (MINIMUM)
STRUCTURAL SYSTEM	WHEN AXLE LOADS EXCEED THE VALUES LISTED ABOVE, OR WHEN WHEEL SPACING IS LESS THAN c/c SPACING, CONTACT ENGINEER PRIOR TO OPERATING UNAUTHORIZED EQUIPMENT.
SYSTEM OVERSTRENGTH FACTOR, OMEGA	1.1.11 FIELD MODIFICATIONS: MODIFICATIONS OF STRUCTURAL MEMBERS DUE TO MISLOCATION, MISFIT, MECHANICAL INTERFERENCE, OR ANY OTHER CONSTRUCTION ISSUE SHALL NOT BE MADE WITHOUT THE PRIOR APPROVAL OF ENGINEER. NO OPENING SHALL BE PLACED IN ANY STRUCTURAL MEMBER UNLESS SHOWN ON THE CONTRACT STRUCTURAL DRAWINGS OR THE APPROVED SHOP DRAWINGS.
DESIGN CRITERIA: SOIL BEARING CAPACITY	1.1.12 PERMANENT EQUIPMENT: SHALL BE LOCATED ONLY ON THE STRUCTURAL MEMBERS INTENDED TO SUPPORT THIS EQUIPMENT AS SHOWN ON THE CONTRACT DRAWINGS OR THE APPROVED SHOP DRAWINGS. IF STRUCTURAL SUPPORT IS NOT CLEAR, OR A QUESTION ARISES, CONTACT ENGINEER PRIOR TO EQUIPMENT INSTALLATION.
GEOTECHNICAL REPORT: REFER TO GEOTECHNICAL ENGINEERING EXPLORATION AND ANALYSIS, PREPARED BY ILLINOIS DRILLING & TESTING, REPORT # 1363, DATED JANUARY 19, 1968.	2.1 EARTHWORK:
CONCRETE (NORMAL WEIGHT UNO): FOOTINGS AND SUB-SLABS	2.1.1 GEOTECHNICAL REPORT: REFER TO GEOTECHNICAL REPORT INDICATED IN DESIGN CRITERIA FOR SITE CONDITIONS, SUITABLE BEARING MATERIALS, STRUCTURAL FILL, BACKFILL MATERIALS, COMPACTION REQUIREMENTS AND PROJECT SPECIFICATIONS FOR EARTHWORK NOT SPECIFIED HEREIN.
EXTERIOR SLABS	2.1.2 EXCAVATIONS: ALL UNSUITABLE EXISTING FILL AND TOPSOIL SHALL BE EXCAVATED BELOW FOOTING BEARING AND REPLACED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT RECOMMENDATIONS. IF EXCAVATIONS SHOULD INDICATE A SAFE SOIL BEARING CAPACITY LESS THAN THE DESIGN CRITERIA SOIL BEARING CAPACITY THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY AND FOUNDATION REVISED TO MEET THIS CONDITION.
#3 BARS & LARGER WELDABLE, ASTM A706 GRADE 60	2.1.3 SITE PREPARATION: ALL UNSUITABLE EXISTING FILL AND TOPSOIL SHALL BE EXCAVATED WITHIN THE BUILDING FOOTPRINT AND REPLACED TO FINISHED PAD ELEVATION IN ACCORDANCE WITH THE GEOTECHNICAL REPORT RECOMMENDATIONS. PROVIDE COMPACTED AGGREGATE SUB-BASE AND VAPOR RETARDER ABOVE PAD AND BELOW SLAB PER SPECIFICATIONS AND GEOTECHNICAL REPORT RECOMMENDATIONS.
HSS TUBE SHAPES, ASTM A500 GRADE B	2.1.4 BACKFILLING: BACKFILL EACH SIDE OF FOUNDATION WALLS IN EQUAL LIFTS. WHERE FINAL GRADES CREATE AN UNBALANCED CONDITION BACKFILL AS FOLLOWS: AT FOUNDATIONS WALLS STRUCTURALLY CONNECTED TO SLABS (SUCH AS DOCK WALLS), BRACE TOP OF WALL UNTIL SLAB IS IN PLACE AND CURED 7 DAYS MINIMUM. AT BASEMENT WALLS DO NOT BACKFILL UNTIL FIRST FLOOR CONSTRUCTION IS COMPLETE OR TOP OF WALLS ARE BRACED. WHERE WALLS ARE NOT STRUCTURALLY CONNECTED AT THE TOP (SUCH AS RETAINING WALLS) BRACING IS NOT REQUIRED.
$E = 1,500,000 \ PSI$ JOISTS - DOUGLAS FIR-LARCH	BRACING 15 NOT REGULARD.
STUDS, PLATES -SPRUCE-PINE-FIR	
POSTS (5x5 AND LARGER) - DOUGLAS FIR-LARCH NO.1 F_b = 1200 PSI F_c = 1000 PSI F_c = 1000 PSI F_c = 1000 PSI F_c = 1,600,000 PSI WOOD TRUSSES - MACHINE STRESS RATED (G = .42 MIN) F_b = 1650 PSI F_t = 1020 PSI	
$F_{\text{c}} = 1700 \text{ PSI}$ $E = 1,500,000 \text{ PSI}$ $LVL \text{ (MICROLLAM OR EQUAL)} \dots \dots$	
$F_{\nu} = 285 \text{ PSI}$ $E = 1,900,000 \text{ PSI}$ $PSL \text{ (PARALLAM OR EQUAL) - BEAMS } \dots $	
$F_{\rm V} = 290 \ \text{PSI}$ $E = 2,000,000 \ \text{PSI}$ $PSL \ (PARALLAM \ OR \ EQUAL) \ - \ COLUMNS/POSTS \ . \ . \ . \ . \ . \ . \ . \ . \ . \ $	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	
FASTENERS: E = 1,800,000 PSI	
WELDS	

1.1.5 EXECUTION: CONTRACTOR TO CROSS CHECK DIMENSIONS, ELEVATIONS, SECTIONS AND

1.1.6 PROJECT CONDITIONS: ALL EXISTING BUILDING DIMENSIONS AND CONDITIONS MUST BE

1.1.7 SHOP DRAWINGS/SUBMITTALS: SHALL BE SUBMITTED BY THE GENERAL CONTRACTOR TO THE ARCHITECT/ENGINEER FOR APPROVAL BEFORE FABRICATION MAY PROCEED.

5	EXECUTION: CONTRACTOR TO CROSS CHECK DIMENSIONS, ELEVATIONS, SECTIONS AND DETAILS BETWEEN ARCHITECTURAL, MECHANICAL AND STRUCTURAL PLANS. AMBROSE	3.1	CONCRETE:
	ENGINEERING IS TO BE NOTIFIED OF ANY VARIANCE THAT WILL AFFECT THE STRUCTURAL FRAMING BEFORE CONTRACTOR BEGINS WORK. ALL EQUIPMENT SUPPORTS AND ANCHORAGES TO BE CROSS CHECKED WITH MANUFACTURER'S DRAWINGS. CONTRACTORS SHALL VERIFY ALL	3.1.1	REFERENCES: CONCRETE CONSTRUCTION SHALL COMPLY WITH THE FOLLOWING STANDARDS AND AS MODIFIED HEREIN:
	PROFILES, HEIGHTS AND DIMENSIONS AT PROJECT SITE PRIOR TO FABRICATION OF ANY MATERIAL AND INFORM THE ENGINEER OF ANY DISCREPANCIES OR FRAMING INTERFERENCES.		ACI 117 "SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS"
6	PROJECT CONDITIONS: ALL EXISTING BUILDING DIMENSIONS AND CONDITIONS MUST BE FIELD VERIFIED PRIOR TO FABRICATION. AMBROSE ENGINEERING SHALL NOT BE RESPONSIBLE FOR ANY EXISTING INFORMATION SUPPLIED BY THE OWNER/ARCHITECT NOR BE LIABLE FOR THOSE EXISTING CONDITIONS THAT VARY FROM THE PREVIOUSLY GIVEN INFORMATION.		ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE" ACI SP-66 "ACI DETAILING MANUAL" ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" ACI 347 "GUIDE TO FORMWORK FOR CONCRETE" ACI 360 "GUIDE TO DESIGN OF SLABS-ON-GROUND" CRSI "MANUAL OF STANDARD PRACTICE" CRSI "PLACING REINFORCING BARS"
7	SHOP DRAWINGS/SUBMITTALS: SHALL BE SUBMITTED BY THE GENERAL CONTRACTOR TO THE ARCHITECT/ENGINEER FOR APPROVAL BEFORE FABRICATION MAY PROCEED. SHOP DRAWINGS/SUBMITTALS SHALL BE PROVIDED FOR THE FOLLOWING COMPONENTS: CONCRETE DESIGN MIXES, REINFORCING STEEL, PRECAST CONCRETE PLANK, PRECAST CONCRETE WALL PANELS, STRUCTURAL PRECAST CONCRETE, TILT-UP WALL PANELS, METAL	3.1.2	MATERIALS: PROPORTION CONCRETE MATERIALS TO ATTAIN 28 DAY CONCRETE MIX DESIGN STRENGTHS INDICATED IN THE DESIGN CRITERIA. SEE SPECIFICATIONS FOR ADDITIONAL MATERIAL REQUIREMENTS.
	BUILDING, STRUCTURAL STEEL, STEEL JOISTS AND GIRDERS, STEEL DECKING, METAL FABRICATIONS, COLD-FORMED STEEL FRAMING, PRE-ENGINEERED COLD-FORMED STEEL TRUSSES, PRE-ENGINEERED WOOD TRUSSES, AND GLUE LAMINATED LUMBER. SEE SPECIFIC MATERIALS SECTIONS FOR ADDITIONAL INFORMATION.	3.1.3	SHOP DRAWINGS/SUBMITTALS: SUBMIT CONCRETE MIX DESIGNS, COMPRESSIVE STRENGTH TEST HISTORY, CEMENT, FLY ASH, AGGREGATE TEST REPORTS, ADMIXTURES, FIBER REINFORCING, REBAR PLACEMENT AND FABRICATION PLANS, LAP LENGTHS, REBAR BENDING DIAGRAMS, AND ALL DETAILS AS REQUIRED TO COMPLETE INSTALLATION.
	NOTES: - GENERAL CONTRACTOR SHALL REVIEW AND STAMP SHOP DRAWINGS BEFORE SUBMITTAL TO ARCHITECT/ENGINEER SUBMIT (2) COPIES TO ARCHITECT/ENGINEER FOR REVIEW. (1) REVIEWED COPY WILL	3.1.4	ACCESSORIES: ALL CONCRETE ACCESSORIES SUCH AS CHAIRS, TIES, ETC., THAT COME IN CONTACT WITH FORMWORK ON EXPOSED CONCRETE SHALL BE GALVANIZED OR PLASTIC COATED. CONCRETE BLOCK OR CLAY MASONRY SHALL NOT BE USED AS CHAIRS FOR SUPPORT OF SLAB-ON-GRADE REINFORCEMENT.
	BE RETURNED. ANY ADDITIONAL SHOP DRAWING COPIES WILL NOT BE RETURNED. - ALL SHOP DRAWINGS SHALL CONTAIN THE ISSUE DATE INDICATED ON THE CONSTRUCTION DOCUMENTS, ALONG WITH ANY ADDENDUMS OR REVISION DATES. - COPIES OF THE STRUCTURAL DRAWINGS SUBMITTED AS SHOP DRAWINGS WILL BE REJECTED.	3.1.5	WELDED WIRE REINFORCEMENT: PROVIDE WELDED WIRE REINFORCEMENT IN ACCORDANCE WITH THE DESIGN CRITERIA. WELDED WIRE REINFORCEMENT SHALL BE FLAT SHEETS ONLY, LAPPED 6" MINIMUM AND POSITIONED AT MID-HEIGHT OF THE SLAB THICKNESS, UNO.
	- ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS SHALL BE NOTED (CLOUD, NOTE, ETC.) ON THE SHOP DRAWINGS SUBMITTED FOR APPROVAL ANY CHANGES ON RESUBMITTED SHOP DRAWINGS SHALL BE CLOUDED.	3.1.6	SYNTHETIC FIBER REINFORCEMENT: PROVIDE SYNTHETIC FIBER REINFORCEMENT IN ACCORDANC WITH THE PROJECT SPECIFICATIONS AT THE DOSAGE RATE INDICATED ON THE PLANS.
8	DEFERRED COMPONENT SUBMITTALS: SHALL BE SUBMITTED BY THE GENERAL CONTRACTOR TO THE ARCHITECT/ENGINEER PRIOR TO CONSTRUCTION. DEFERRED SUBMITTALS SHALL BE	3.1.7	BAR REINFORCEMENT: PROVIDE BAR REINFORCEMENT IN ACCORDANCE WITH THE DESIGN CRITERIA. WHEN BAR REINFORCEMENT IS CALLED FOR IN A CERTAIN PORTION OF THE BUILDING, IT SHALL BE DUPLICATED IN SIMILAR PORTIONS OF THE BUILDING, UNO.
	PROVIDED FOR THE FOLLOWING COMPONENTS: PRECAST CONCRETE PLANK, PRECAST CONCRETE WALL PANELS, STRUCTURAL PRECAST CONCRETE, TILT-UP WALL PANELS, METAL BUILDING, STEEL JOISTS AND GIRDERS, COLD-FORMED STEEL, WOOD TRUSSES, AND GLUE LAMINATED LUMBER.	3.1.8	MINIMUM COVER: INSTALL BAR REINFORCEMENT WITH THE FOLLOWING MINIMUM COVER UNLESS A GREATER COVER IS REQUIRED DUE TO FIRE PROTECTION:
	NOTES:		CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH
	- GENERAL CONTRACTOR SHALL REVIEW AND STAMP THE DEFERRED SUBMITTALS BEFORE SUBMITTAL TO ARCHITECT/ENGINEER SUBMIT (4) COPIES OF DRAWINGS AND CALCULATIONS TO ENGINEER. (3) COPIES WILL BE RETURNED WITH ENGINEER'S APPROVAL STAMP.		#5 BAR AND SMALLER
	- ALL COMPONENT SUBMITTALS SHALL BEAR AN ORIGINAL SEAL AND SIGNATURE OF THE COMPONENT DESIGNER ALL SUBMITTED COPIES MUST BE THE FINAL "FIELD USE" SETS WHICH INCLUDES ALL		#11 BAR AND SMALLER
9	CORRECTIONS MADE DUE TO SHOP DRAWING REVIEW COMMENTS. SPECIAL INSPECTIONS: AN INSPECTION & TESTING COMPANY SHALL BE RETAINED IN ACCORDANCE WITH THE IBC FOR THE FOLLOWING: - SOILS AND EARTHWORK SUPPORTING FOUNDATIONS AND SLABS.	3.1.9	DEVELOPMENT: THE MINIMUM DEVELOPMENT LENGTH OF NON-CONTINUOUS BAR REINFORCEMENT IS SHOWN BELOW. TERMINATE WITH A STANDARD HOOK IN ACCORDANCE WITH ACI IF REQUIRED DEVELOPMENT LENGTH CANNOT BE OBTAINED. THE LAP SPLICE LENGTH OF CONTINUOUS BAR REINFORCEMENT IS SHOWN BELOW. IN GROUPS OF PARALLEL BARS, LAP SPLICES SHALL BE STAGGERED.
	- CONCRETE TEST CYLINDERS AND STRENGTH TESTING CONCRETE REINFORCEMENT MASONRY PER ACI 530.1-05/ASCE 5-05/TMS 402-05/- SPECIFICATION FOR MASONRY		DEVELOPMENT LENGTH LAP SPLICE LENGTH f'o = 3000 PSI, GRADE 60, UNCOATED: ALL BARS (EXCEPT TOP HORIZ. BARS):
	STRUCTURES TABLE 4 - LEVEL B QUALITY ASSURANCE STRUCTURAL STEEL WELDED AND BOLTED CONNECTIONS JOIST AND JOIST GIRDER WELDED AND BOLTED CONNECTIONS.		#6 BAR AND SMALLER
10	- METAL DECK FASTENING POST INSTALLED EXPANSION AND EPOXY ANCHORS. CONSTRUCTION LOADS: PLACEMENT OF CONSTRUCTION EQUIPMENT, MATERIALS, AND PERSONNEL	7	#6 BAR AND SMALLER 57 BAR DIAMETERS 74 BAR DIAMETERS #7 BAR AND LARGER 71 BAR DIAMETERS 93 BAR DIAMETERS f'c = 4000 PSI, GRADE 60, UNCOATED: ALL BARS (EXCEPT TOP HORIZ. BARS):
10	SHALL NOT EXCEED THE DESIGN LIVE LOAD OF THE STRUCTURE. CONCRETE SHALL CURE A MINIMUM OF 7 DAYS BEFORE THE APPLICATION OF CONSTRUCTION LOADS. IN ADDITION, EQUIPMENT PLACED ON FLOORS SHALL ALSO COMPLY WITH THE FOLLOWING:		#6 BAR AND SMALLER
	FORKLIFT SPACING BETWEEN WHEELS ON AXLE IS NOT LESS THAN 38" c/c OR SCISSOR LIFT SPACING BETWEEN WHEELS ON AXLE IS NOT LESS THAN 25" c/c.		#6 BAR AND SMALLER 50 BAR DIAMETERS 64 BAR DIAMETERS 87 BAR AND LARGER 62 BAR DIAMETERS 80 BAR DIAMETERS
	SLAB-ON-GRADE: ALLOWABLE EQUIPMENT AXLE LOADS SLAB DEPTH FORKLIFT SCISSOR/PLATFORM LIFTS 4" NONE 5,400 LBS	3.1.10	HOT WEATHER CONCRETING: FOLLOW ACI 305 "HOT WEATHER CONCRETING" WHEN MAXIMUM DAILY TEMPERATURE EXCEEDS 85°F, OR RAPID DRYING CONDITIONS EXIST; EVAPORATION RATE > .15 LB/SF/HR PER FIGURE 2.1.5.
	5" 8,600 LBS 6,800 LBS >=6" 11,400 LBS 9,200 LBS MODULUS OF SUBGRADE, k (MINIMUM) 100 PCI	3.1.11	COLD WEATHER CONCRETING; FOLLOW ACI 306 "COLD WEATHER CONCRETING" WHEN FREEZING CONDITIONS OR MEAN DAILY TEMPERATURE FALLS BELOW 40°F.
	WHEN AXLE LOADS EXCEED THE VALUES LISTED ABOVE, OR WHEN WHEEL SPACING IS LESS THAN c/c SPACING, CONTACT ENGINEER PRIOR TO OPERATING UNAUTHORIZED EQUIPMENT.	3.1.12	SLABS: MAY BE POURED AS A CONTINUOUS SCREEDED POUR WITH SAW CUT CONTROL JOINTS IN BOTH DIRECTIONS. SAW CUTS 70 BE MADE WITHIN 8 HOURS OF POUR AND SPACED AS INDICATED IN THE DRAWINGS.
11	FIELD MODIFICATIONS: MODIFICATIONS OF STRUCTURAL MEMBERS DUE TO MISLOCATION, MISFIT, MECHANICAL INTERFERENCE, OR ANY OTHER CONSTRUCTION ISSUE SHALL NOT BE MADE WITHOUT THE PRIOR APPROVAL OF ENGINEER. NO OPENING SHALL BE PLACED IN ANY STRUCTURAL MEMBER UNLESS SHOWN ON THE CONTRACT STRUCTURAL	3.1.13	WALLS: MAXIMUM POUR LENGTH 100 FT BETWEEN FORMED CONSTRUCTION JOINTS. FOR WALLS EXPOSED TO VIEW PROVIDE INTERMEDIATE CONTROL JOINTS NO GREATER THAN 30 FT ON CENTER. JOINTS SHOULD ALIGN WITH BUILDING CONTROL JOINTS WHEN PRESENT AND COORDINATED WITH ARCHITECTURAL DRAWINGS
	DRAWINGS OR THE APPROVED SHOP DRAWINGS.	3.1.14	OPENINGS: CONTRACTOR TO PROVIDE AND COORDINATE WITH ALL OTHER TRADES FOR SIZE AND LOCATIONS OF ANY AND ALL OPENINGS. SLEEVES, E.C. OCCURRING IN WALLS

AND LOCATIONS OF ANY AND ALL OPENINGS, SLEEVES, ETC. OCCURR FOOTINGS, AND FLOORS. SLEEVE LAYOUTS SHALL BE SUBMITTED FO

3.1.15 BOND BREAKER: PROVIDE BOND BREAKER MATERIAL WHERE SLABS ABUT WALLS.

OTHER VERTICAL SURFACES.

6.1 WOOD CONSTRUCTION:

NDS COMMENTARY

NDS SUPPLEMENTS

BOLT DIAMETER

PLYWOOD AND ORIENTATED STRAND BOARD: AMERICAN PLYWOOD ASSOCIATION

SOAP MAY BE USED TO LUBRICATE SCREWS.

NDS GUIDELINES

6.1.1 REFERENCES: ALL WOOD CONSTRUCTION SHALL BE EXECUTED IN CONFORMANCE WITH

NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION

6.1.3 ALL WOOD IN DIRECT CONTACT WITH EARTH OR CONCRETE SHALL BE PRESSURE

6.1.4 BEARING AND SHEAR WALLS TO HAVE DOUBLE TOP PLATE, LAPPED AT WALL AND

6.1.5 HOLES FOR BOLTS IN WOOD SHALL BE BORED WITH A BIT OF THE SAME NOMINAL

6.1.7 LAG SCREWS AND WOOD SCREWS SHALL BE SCREWED AND NOT DRIVEN INTO PLACE.

MACHINE WASHER

3" DIA x 7/16"

6.1.9 LAY ALL STRUCTURAL PLYWOOD ON ROOF AND FLOORS WITH FACE GRAIN

6.1.10 CONNECTOR HARDWARE MODEL NUMBERS ARE THOSE FOR THE SIMPSON STRONG-TIE

6.1.11 ALL FASTENERS FOR PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD

4" DIA x 1/2"

OFFICIALS (ICBO) ACCEPTANCE MAY BE SUBSTITUTED.

THE REQUIREMENTS OF THE FOLLOWING STANDARDS AND AS MODIFIED HEREIN:

PLATE CONNECTED WOOD TRUSSES AND PRE-ENGINEERED METAL CONNECTORS:

SAWN LUMBER, GLUE-LAMINATED TIMBER, WOOD I JOIST, COMPOSITE LUMBER, METAL

ALLOWABLE STRESS DESIGN (ADS) MANUAL FOR ENGINEERED WOOD CONSTRUCTION

MATERIALS: CONTRACTOR SHALL SELECT LUMBER SPECIES AND GRADE CONFORMING WITH THE STRENGTH REQUIREMENTS SPECIFIED UNDER THE GENERAL DESIGN

PARTITION INTERSECTION WITH 3 - 16d NAILS. STAGGER SPLICES OF UPPER AND

HOLES FOR LAG SCREWS SHALL BE FIRST BORED TO THE SAME NOMINAL DIAMETER

ALL BOLTS AND LAG SCREWS SHALL BE PROVIDED WITH METAL WASHERS UNDER HEADS AND NUTS WHICH BEAR ON WOOD. (APPLIES ALSO TO INSERTED EXPANDING

PERPENDICULAR TO SUPPORTS UNLESS NOTED OTHERWISE. STAGGER INTERIOR PANEL

COMPANY. EQUIVALENT CONNECTORS WITH INTERNATIONAL CONFERENCE OF BUILDING

SHALL BE OF HOT-DIPPED ZINC-COATED GALVANIZED STEEL OR STAINLESS STEEL.

2-3/4" DIA x 5/16" 2-1/2" x 2-1/2" x 1/4"

3-1/2" DIA x 7/16" 3-1/2" x 3-1/2" x 3/8"

STEEL PLATE WASHER

3-3/4" x 3-3/4" x 3/8"

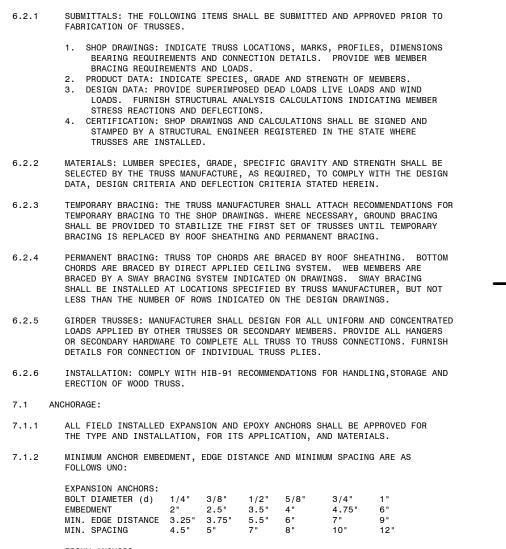
3" x 3" x 5/16"

AND DEPTH AS THE SHANK AND THE REST NO LARGER THAN THE ROOT OF THE

6.1.12	ALL NAILS FOR STRUCTURAL WORK SHALL BE COMMON WIRE NAILS CONFORMING TO THE FOLLOWING MINIMUM SIZES:
	8d
6.1.13	CONNECTIONS: WHERE SPECIAL NAILING IS NOT INDICATED ON DRAWINGS, MEMBERS SHALL BE CONNECTED IN ACCORDANCE WITH THE FOLLOWING SCHEDULE:
	1. JOISTS OR RAFTERS TO SIDES OF STUDS 8" JOISTS OR LESS
	2. STUDS TO BEARINGS 2x4 STUDS TN EACH SIDE
	3. BLOCKING BETWEEN JOISTS, TN EACH SIDE EACH END
	4. LEDGERS TO STUDS 1" LEDGER
	5. DOUBLE TOP PLATES LOWER PLATE TO 2x4 STUD
	6. SILL PLATES TO PARALLEL FRAMING
	7. MULTIPLE STUDS STAGGER FOR WIDTHS MORE THAN 4"
	8. BUILT UP BEAMS 10" OR LESS IN DEPTH EACH SIDE STAGGERED 16d @ 12" OC MORE THAN 10" IN DEPTH
	9. STRUCTURAL PLYWOOD AT SUPPORTED EDGES
	10. RAFTERS OR WOOD TRUSSES - SIMPSON MODEL H1 ANCHOR TO ALL BEARING

	0.2.1	FABRICATION OF TRUSSES.
SHORTS 0.148" DIAMETER x 1-5/8" PLUS THICKNESS OF SPLICE		 SHOP DRAWINGS: INDICATE TRUSS LOCATIONS, MARKS, PROFILES, DIMEN BEARING REQUIREMENTS AND CONNECTION DETAILS. PROVIDE WEB MEMB BRACING REQUIREMENTS AND LOADS. PRODUCT DATA: INDICATE SPECIES, GRADE AND STRENGTH OF MEMBERS.
ECTIONS: WHERE SPECIAL NAILING IS NOT INDICATED ON DRAWINGS, MEMBERS L BE CONNECTED IN ACCORDANCE WITH THE FOLLOWING SCHEDULE:		3. DESIGN DATA: PROVIDE SUPERIMPOSED DEAD LOADS LIVE LOADS AND WIN LOADS. FURNISH STRUCTURAL ANALYSIS CALCULATIONS INDICATING ME STRESS REACTIONS AND DEFLECTIONS.
JOISTS OR RAFTERS TO SIDES OF STUDS 8" JOISTS OR LESS		 CERTIFICATION: SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED A STAMPED BY A STRUCTURAL ENGINEER REGISTERED IN THE STATE WHERE TRUSSES ARE INSTALLED.
. ,	6.2.2	MATERIALS: LUMBER SPECIES, GRADE, SPECIFIC GRAVITY AND STRENGTH SHA
STUDS TO BEARINGS 2x4 STUDS TN EACH SIDE		SELECTED BY THE TRUSS MANUFACTURE, AS REQUIRED, TO COMPLY WITH THE DATA, DESIGN CRITERIA AND DEFLECTION CRITERIA STATED HEREIN.
2x6 & 2x8 STUDS TN EACH SIDE	6.2.3	TEMPORARY BRACING: THE TRUSS MANUFACTURER SHALL ATTACH RECOMMENDATI TEMPORARY BRACING TO THE SHOP DRAWINGS. WHERE NECESSARY, GROUND BRA SHALL BE PROVIDED TO STABILIZE THE FIRST SET OF TRUSSES UNTIL TEMPO
BLOCKING BETWEEN JOISTS, TN EACH SIDE EACH END 2 - 10d		BRACING IS REPLACED BY ROOF SHEATHING AND PERMANENT BRACING.
RAFTERS, OR STÚDS IN PLACE OF TN 2 - 16d BETWEEN JOIST OR RAFTER BRGS (PLATES) TN EACH SIDE , 3 - 10d	6.2.4	PERMANENT BRACING: TRUSS TOP CHORDS ARE BRACED BY ROOF SHEATHING. CHORDS ARE BRACED BY DIRECT APPLIED CEILING SYSTEM. WEB MEMBERS AR BRACED BY A SWAY BRACING SYSTEM INDICATED ON DRAWINGS. SWAY BRACIN
LEDGERS TO STUDS 1" LEDGER		SHALL BE INSTALLED AT LOCATIONS SPECIFIED BY TRUSS MANUFACTURER, BU LESS THAN THE NUMBER OF ROWS INDICATED ON THE DESIGN DRAWINGS.
DOUBLE TOP PLATES LOWER PLATE TO 2x4 STUD	6.2.5	GIRDER TRUSSES: MANUFACTURER SHALL DESIGN FOR ALL UNIFORM AND CONCE LOADS APPLIED BY OTHER TRUSSES OR SECONDARY MEMBERS. PROVIDE ALL HA OR SECONDARY HARDWARE TO COMPLETE ALL TRUSS TO TRUSS CONNECTIONS. F DETAILS FOR CONNECTION OF INDIVIDUAL TRUSS PLIES.
SILL PLATES TO PARALLEL FRAMING	6.2.6	INSTALLATION: COMPLY WITH HIB-91 RECOMMENDATIONS FOR HANDLING, STORA ERECTION OF WOOD TRUSS.
TO CONCRETE FOUNDATION WALLS «" DIAMETER ANCH. @ 4' OC MIN TO MASONRY FOUNDATION WALLS «" DIAMETER ANCH. @ 4' OC MIN	7.1	ANCHORAGE:
MULTIPLE STUDS STAGGER FOR WIDTHS MORE THAN 4"	7.1.1	ALL FIELD INSTALLED EXPANSION AND EPOXY ANCHORS SHALL BE APPROVED F THE TYPE AND INSTALLATION, FOR ITS APPLICATION, AND MATERIALS.
	7.1.2	MINIMUM ANCHOR EMBEDMENT, EDGE DISTANCE AND MINIMUM SPACING ARE AS
BUILT UP BEAMS 10" OR LESS IN DEPTH EACH SIDE STAGGERED 16d @ 12" OC		FOLLOWS UNO:
MORE THAN 10" IN DEPTH		EXPANSION ANCHORS: BOLT DIAMETER (d) 1/4" 3/8" 1/2" 5/8" 3/4" 1" EMBEDMENT 2" 2.5" 3.5" 4" 4.75" 6"
STRUCTURAL PLYWOOD AT SUPPORTED EDGES		MIN. EDGE DISTANCE 3.25" 3.75" 5.5" 6" 7" 9" MIN. SPACING 4.5" 5" 7" 8" 10" 12"
RAFTERS OR WOOD TRUSSES - SIMPSON MODEL H1 ANCHOR		EPOXY ANCHORS: BOLT DIAMETER (d) 3/8" 1/2" 5/8" 3/4" 1"
TO ALL BEARING		EMBEDMENT 3.5" 4.25" 5" 6.62" 8.25" MIN. EDGE DISTANCE 6" 6.5" 7.5" 10" 12.5"
10 MARTIEN		MIN. SPACING 5.25" 6.5" 7.5" 10" 12.5"

6.2 PRE-ENGINEERED WOOD TRUSSES:



STRUCTURAL SHEET INDEX

GENERAL NOTES FOUNDATION PLAN ROOF FRAMING PLAN

SECTIONS & DETAILS

Sheet Name

Sheet Number

BLAKEMORE

ARCHITECTS 400 N. First Street Rockford, IL 61107

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New Utility Building



3501 Kishwaukee Street Rockford, Illinois

THE CONTRACTOR SHALL DETERMINE EXACT DIMENSIONS AND CONDITIONS AT THE SITE PRIOR TO SUBMITTING A BID. THE CONTRACTOR SHALL COORDINATE ALL DRAWINGS WITH ACTUAL FIELD CONDITIONS PRIOR TO PROCEEDING WITH THE WORK AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES. THIS DRAWING IS THE PROPERTY OF McCLELLAN BLAKEMORE ARCHITECTS AND MAY NOT BE REPRODUCED WITHOUT THE PRIOR WRITTEN PERMISSION OF THE ARCHITECT.

NO.	DATE	DESCRIPTION
1.	06-26-202	nitial Layout
2.	07-7-2020	Review Set
3.	07-16-202	Review Set
4.	08-3-2020	Issued for Construction
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BA Project No.

Scale

12" = 1'-0"

20-18

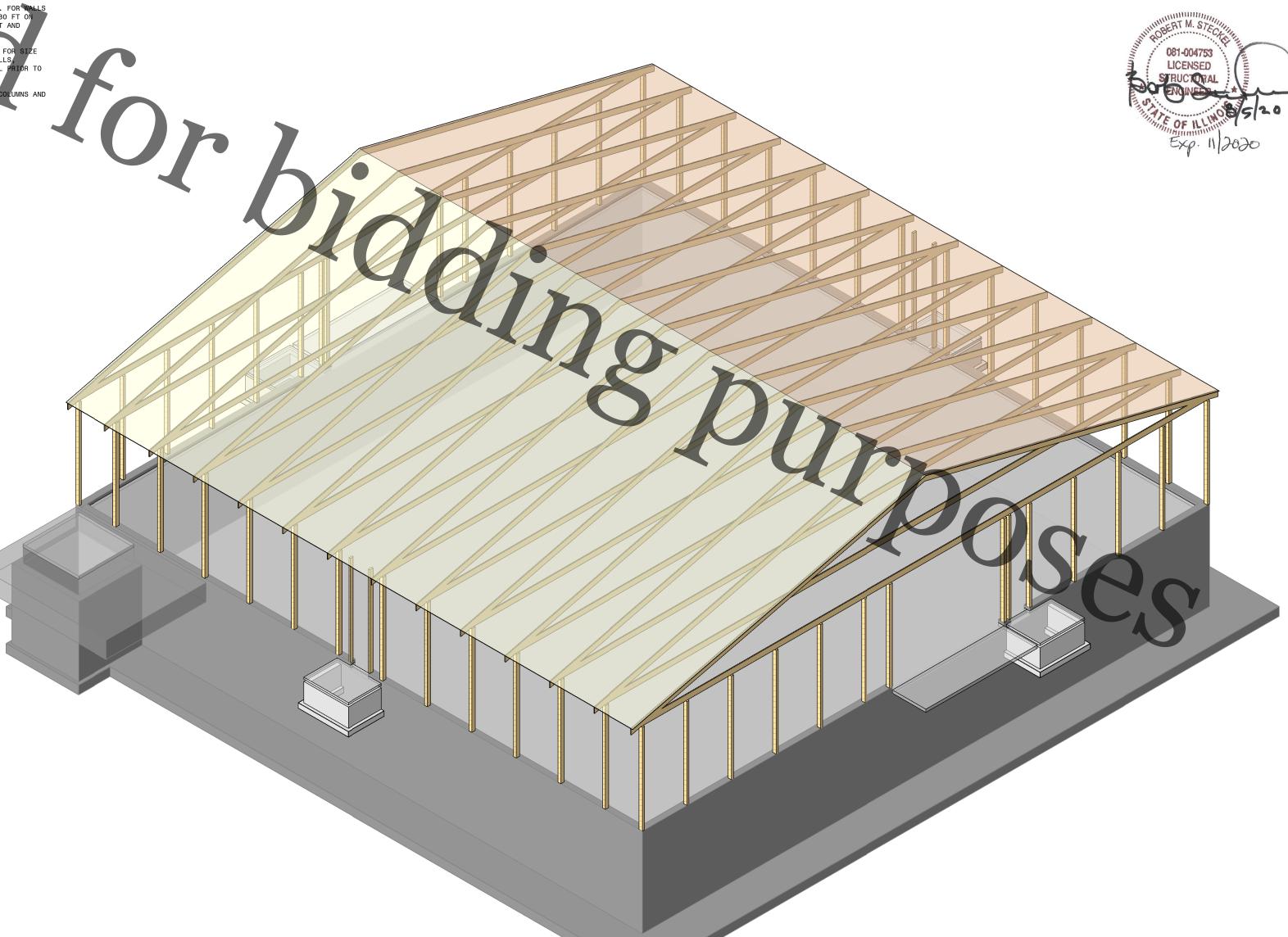
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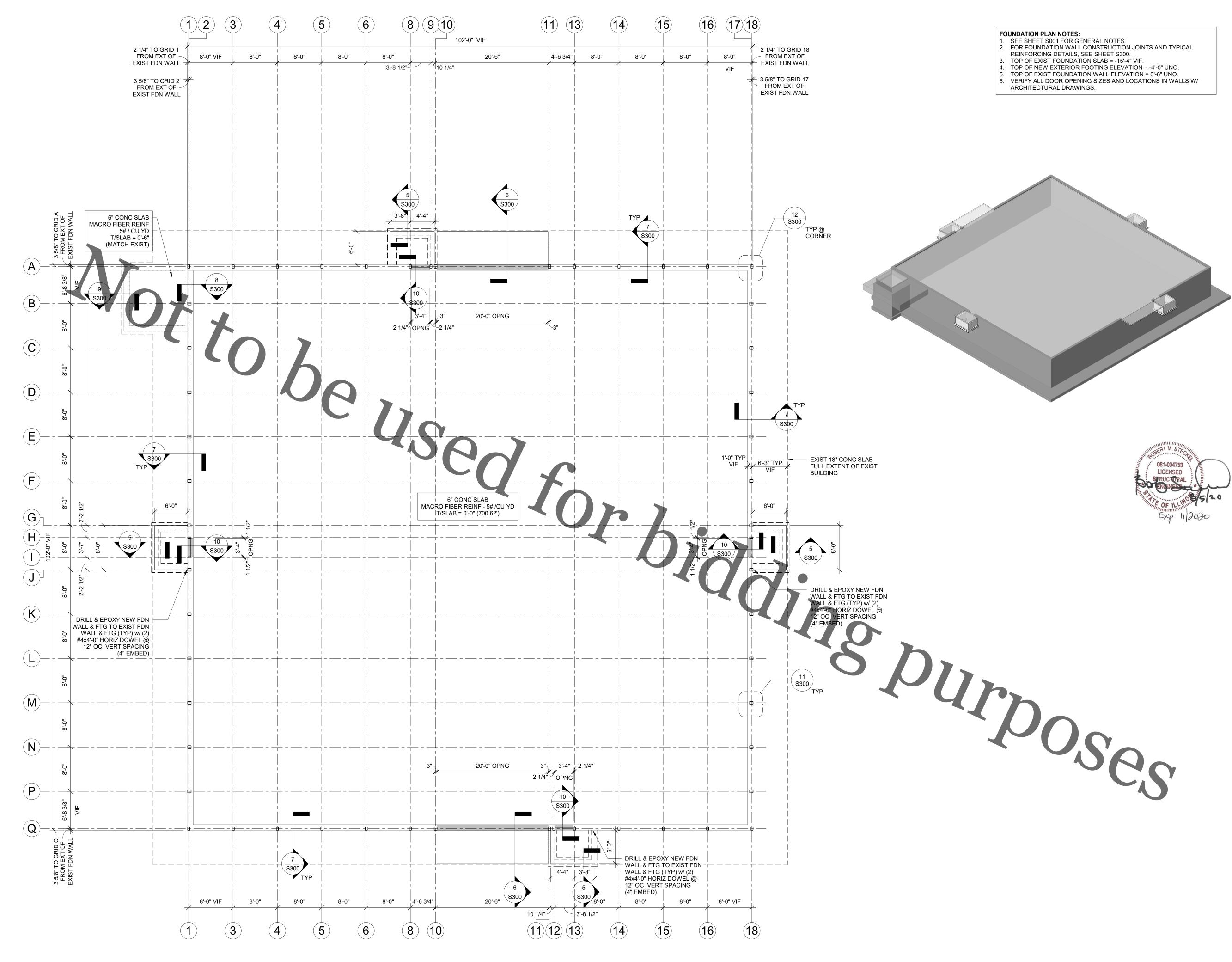
GENERAL NOTES

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Sheet No. **S001**

PROFESSIONAL DESIGN FIRM REGISTRATION # 184-003342





FOUNDATION PLAN

S100 SCALE: 1/8" = 1'-0'



Telephone: 815-227-0023 Email: Brian@blakemore-architects.com Web: www.blakemore-architects.com

400 N. First Street Rockford, IL 61107



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for



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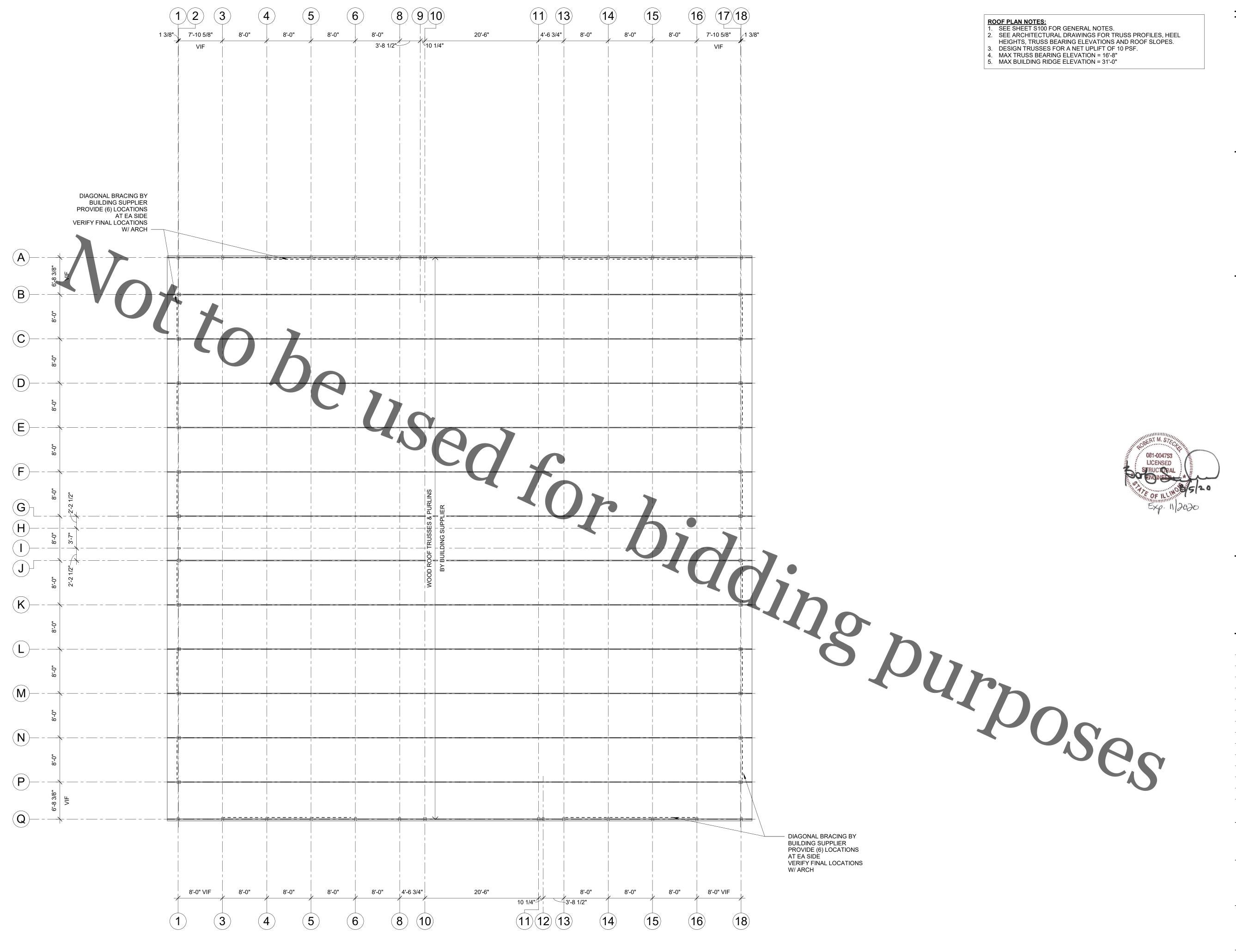
FOUNDATION PLAN

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S100

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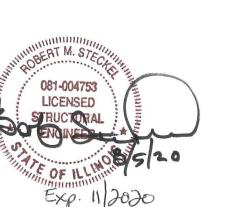
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815-227-0023 Telephone: Email: Brian@blakemore-architects.com Web: www.blakemore-archtects.com



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ROOF FRAMING PLAN

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S101

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